

Location Restrictions Demonstrations

DTE Electric Company River Rouge Power Plant Bottom Ash Basin Coal Combustion Residual Unit

> 1 Belanger Park Drive River Rouge, Michigan

October 2018



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Prepared For DTE Electric Company

Graham Crockford, C.P.G. Senior Project Geologist

David B. McKenzie, P.E. Senior Project Engineer

TRC | DTE Electric Company

Final

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I, the undersigned Michigan Professional Engineer, hereby certify that I am familiar with the technical requirements of Title 40 Code of Federal Regulations Part 257 Subpart D (§257). I also certify that it is my professional opinion that, to the best of my knowledge, information, and belief, that the information in this demonstration is in accordance with current good and accepted engineering practice(s) and standard(s) and meets the requirements of §257.60 through §257.64.

For the purpose of this document, "certify" and "certification" shall be interpreted and construed to be a "statement of professional opinion." The certification is understood and intended to be an expression of my professional opinion as a Michigan Licensed Professional Engineer, based upon knowledge, information, and belief. The statement(s) of professional opinion are not and shall not be interpreted or construed to be a guarantee or a warranty of the analysis herein.



David B McKenzie, P.E.

License No: 6201042332

Section 1 Background

The purpose of this document is to determine whether the Coal Combustion Residual (CCR) Bottom Ash Basin (BAB) at the River Rouge Power Plant (RRPP) is in compliance with the location restrictions outlined in the Environmental Protection Agency's (EPA) final CCR rule [Title 40 Code of Federal Regulations Parts 257 and 261] Subpart D – "Standards for the Disposal of Coal Combustion Residuals in Landfills and Surface Impoundments" (§257.60 through §257.64, federal rule). The BAB is considered a CCR surface impoundment according to the federal rule (§257.53).

This document includes information from a desktop study and well installation activities, and also includes engineering calculations to demonstrate that the BAB complies with placement above the uppermost aquifer criteria (§257.60), and with location criteria with respect to wetlands (§257.61), fault areas (§257.62), seismic impact zones (§257.63), and unstable areas (§257.64).

Supporting documents are provided in appendices to this demonstration.

1.1 Facility and CCR Unit Information

The RRPP property is located at the confluence of the Rouge River and the Detroit River, at 1 Belanger Park Drive, within the City of River Rouge in Wayne County, Michigan. The RRPP was constructed in the early 1950s, just northeast of the DTE Electric RRPP. The BAB CCR unit was originally constructed during the power plant construction by incising 2H:1V side slopes into native site soils. In 1998, the impoundment was reconstructed in place by installing steel sheet pile walls around the perimeter of the BAB to an approximate depth of 30 feet below ground surface (ft bgs). The tip of the perimeter wall was installed to an elevation of 550 ft MSL and the top of the wall was cut at 580.33 ft MSL. The steel basin walls are tied back on 20-ft centers to a 10-ft deep secondary sheet pile anchor wall to maximize the stability of the basin perimeter sheeting. The base of the basin is maintained at a depth of 569 ft MSL by periodically dredging accumulated CCR.

The BAB is used for receiving sluiced bottom ash and other process flow effluent pumped from the power plant to the eastern end of the BAB. There is a sheet pile weir near the middle of the BAB that maintains the water elevation in the eastern portion to approximately 577.5 feet through gravity flow. The water in the western portion of the BAB is maintained at an elevation of no higher than 577 feet. Water in the western portion of the BAB is either recirculated back to the RRPP, discharged into the Detroit River in accordance with a National Pollution Discharge Elimination System (NPDES) permit, or may be discharged under permit to the City of River

Rouge sewer system for treatment at Wayne County's Downriver Wastewater Treatment Facility (DWTP). The CCR retained in the BAB is routinely cleaned out and either sold for beneficial reuse or is disposed at the Sibley Quarry Landfill (SQL).

1.2 Site Setting

The RRPP BAB CCR unit is located immediately adjacent to the Rouge River near the intersection of the Rouge River and Detroit River (Figure 1). A groundwater monitoring system has been established for the RRPP BAB CCR unit as detailed in the *Groundwater Monitoring System Summary Report – DTE Electric Company River Rouge Power Plant Bottom Ash Basin Coal Combustion Residual Unit* (GWMS Report) (TRC, October 2017). The detection monitoring well network for the BAB CCR unit currently consists of five monitoring wells that are screened in the uppermost aquifer. The monitoring well boring logs are included in Appendix A.

The RRPP CCR unit is underlain initially by approximately 10 feet of surficial fill of various composition (gravel, sand, silt and clay, brick and/or concrete fragments). The fill is partially saturated in some areas, but is not continuously saturated across the RRPP, does not represent a significant, usable source of water, and is, therefore, not an aquifer. An organic layer is often encountered beneath the surficial fill that is then underlain by a silt/clay-rich unit that ranges from 3 to about 8 feet thick near the BAB. Beneath the silt/clay-rich unit, there is a saturated sand and gravel unit that often coarsens from sand to gravel with depth. This coarse-grained sand and gravel unit is present from as shallow as 15 ft bgs to as deep as 25.5 ft bgs. This same coarse-grained unit is observed in most of the historical boring logs across the RRPP and appears to be a relatively continuous unit across the RRPP. The hydraulic conductivity measured in this unit using single well hydraulic conductivity tests (e.g., slug tests) ranged from 9.5 to 120 feet/day, with a geometric mean of approximately 25.5 feet/day. Based on this information, this coarse-grained sand and gravel unit represents the uppermost aquifer present at the RRPP BAB CCR unit.

The coarse-grained sand and gravel uppermost aquifer is underlain by a more than 60-foot-thick contiguous silty clay-rich deposit that serves as a natural lower confining hydraulic barrier that isolates the uppermost aquifer from the underlying Dundee limestone that represents the next aquifer. There is no apparent hydraulic connection between the uppermost aquifer and the Dundee limestone aquifer, and the limestone aquifer is artesian.

A definitive groundwater flow direction to the northeast with an average gradient of 0.00067 foot/foot (using data from June 2016 through September 2017) within the uppermost aquifer is evident around the RRPP BAB CCR unit, with potential groundwater velocities within the uppermost aquifer ranging from approximately 5.8 to 73 feet/year.

Section 2 Location Restrictions

The location restrictions designated in the federal CCR rule are presented below with a corresponding demonstration to show compliance with each restriction. The location restrictions include placement above the uppermost aquifer, within wetlands, near fault areas, within seismic impact zones, and in unstable areas based on geologic and geomorphologic information. Supporting information for the demonstrations is included in the appendices to this report.

2.1 §257.60 – Placement above the Uppermost Aquifer

The federal CCR rule requires that CCR units such as the RRPP BAB must be constructed with a base that is located no less than 1.52 meters (five feet) above the upper limit of the uppermost aquifer, or must demonstrate that there will not be an intermittent, recurring, or sustained hydraulic connection between any portion of the base of the CCR unit and the uppermost aquifer due to normal fluctuations in the groundwater elevations (including the seasonal high water table). As stated in Section 1.1 (above), the perimeter of each BAB is constructed of steel sheet piling installed to a depth of approximately 30 ft below ground surface. The top of the wall elevation is 580.33 ft MSL and the tip of the wall elevation is 550 ft MSL. The design dredge depth for the basin is 569 ft MSL. The uppermost aquifer is found at an approximate elevation of 563 to 565.5 ft MSL. Cross-sections showing the installation top and bottom elevation of the sheet pile and approximate pond bottom elevation for the BAB are included in Appendix B.

Based on this demonstration, the base of the BAB is not located greater than five feet above the upper limit of the uppermost aquifer; therefore, the BAB is not in compliance with the requirements of §257.60 and is subject to closure requirements under 40 CFR 257.101(b)(1). Because of this condition, DTE Electric, beginning in late 2017, as a presumptive remedy designed, constructed and by March of 2018, initiated the operations of a groundwater capture system around the perimeter of the BAB to effectively control the uncertainty around the potential migration of CCR constituents from the RRPP BAB to groundwater. With this installed system operating continuously since March 2018, the groundwater level of the uppermost aquifer in the area of the BAB has been depressed, thereby maintaining groundwater hydraulic capture within the uppermost aquifer around the BAB.

2.2 §257.61 – Wetlands

The CCR location standards restrict existing and new CCR surface impoundments from being located in wetlands, as defined at 40 CFR 232.2 (40 CFR 257.61(a)). Wetlands are defined in

40 CFR 232.2 definition of *Waters of the United States* (3)(iv) as, "...those areas that are inundated or saturated by surface or groundwater at a frequency and duration sufficient to support, and that under normal circumstances do support, a prevalence of vegetation typically adapted for life in saturated soil conditions. Wetlands generally include swamps, marshes, bogs, and similar areas." TRC reviewed National Wetland Inventory (NWI) Maps and Michigan Resource Information System (MIRIS) Land Cover Maps archived and available through Michigan Department of Natural Resources (MDNR), Michigan Resource Inventory Program (MRIP) to ascertain whether or not the RRPP BAB is located in wetlands.

As shown on the site maps in Appendix C, soils along the Detroit River at the site are designated primarily as fluvial deposits and/or wetland soils. NWI (2005) recognizes one area located approximately ½-mile west of the BAB and along the Rouge River as a wetland. This wetland area is not immediately adjacent to the BAB, and therefore, there is no risk of impact to this area from the BAB operations.

Based on TRC's review of wetland inventory resources and current site conditions, TRC is of the opinion that the RRPP BAB is not located in an area exhibiting wetland characteristics, and any continued operations at the BAB will have no potential to impact any wetlands near the CCR unit. TRC also concludes that, due to their use as NPDES treatment units, these basins are not wetlands, as defined in 40 CFR 232.2.

2.3 §257.62 – Fault areas

The federal CCR rule requires that CCR units not be located within 60 meters (200 feet) of the outermost damage zone of a fault that has had displacement in Holocene time (within the most recent 11,700 years) unless the owner or operator demonstrates that an alternative setback distance of less than 60 meters (200 feet) will prevent damage to the structural integrity of the CCR unit. As shown on the U.S. Quaternary Folds and Faults Database Map (USGS, Accessed 9/7/2018) in Appendix D, no faults have been mapped near the RRPP BAB.

Evidence of active faulting during the Holocene in the RRPP BAB area is not supported by this determination; therefore, the BAB is in compliance with the requirements of §257.62.

2.4 §257.63 – Seismic Impact Zones

The federal CCR rule requires that CCR units not be located in seismic impact zones unless the owner or operator demonstrates that all structural components including liners, leachate collection and removal systems, and surface water control systems, are designed to resist the maximum horizontal acceleration in lithified earth material for the site. The federal CCR rule defines a seismic impact zone as "an area having a 2% or greater probability that the maximum

expected horizontal acceleration, expressed as a percentage of the earth's gravitation pull (g), will exceed 0.10 g in 50 years."

To determine whether the RRPP BAB is located in a seismic impact zone, the USGS Earthquake Hazards Program was consulted to determine the earthquake hazard for the RRPP. The 2015 National Earthquake Hazards Reduction Program U.S. seismic design maps website (USGS 2015; Appendix E) indicates a mapped peak ground acceleration of 0.053 g for the Station area. Using the default site adjustment factor results in a design peak ground acceleration of 0.085 g in 50 years. Since this calculation indicates that the design peak ground acceleration value will not exceed 0.10 g in 50 years, the RRPP BAB is not located in a seismic impact zone, and therefore, the RRPP BAB is in compliance with the requirements of §257.63.

2.5 §257.64 – Unstable Areas

The federal CCR rule requires that CCR units not be located in an unstable area unless the owner or operator demonstrates that recognized and generally accepted good engineering practices have been incorporated into the design of the CCR unit to ensure that the integrity of the structural components of the CCR unit will not be disrupted. Factors associated with soil conditions resulting in significant differential settlement, geologic or geomorphologic features, and human-made features or events must be evaluated to determine compliance. This demonstration was performed by reviewing geotechnical data, local geology and topography, and evaluating human-made features at the BAB area.

The geotechnical exploration performed at the RRPP BAB area identified a mix of silt, clay, sand, gravel, and fill overlying a sand and gravel uppermost aquifer. The uppermost aquifer is underlain be approximately 60 feet of firm silty clay which isolates the uppermost aquifer from the underlying Dundee limestone (Appendix B). These observations suggest that there are no unstable soil or underlying bedrock conditions proximal to the BAB. Additionally, the perimeter walls of the RRPP BAB are constructed of steel sheet pile driven through the surficial fill and sand/gravel and into the underlying clay, and these perimeter walls are tied back to driven steel sheets located 15 feet behind the BAB perimeter walls. These tie-backs further serve to stabilize the BAB walls and minimize potential for sidewall collapse.

Human-made features at the facility include seawall placement along portions of the Detroit River and Rouge River for shoreline protection to further minimize any instability of site soils caused by hydrologic forces from the adjacent water bodies.

Evidence of unstable areas due to soil conditions resulting in significant differential settling, geologic or geomorphologic features, or human-made features or events is not supported by this determination; therefore, the RRPP BAB is not located in an unstable area. The BAB is in compliance with the requirements of §257.64.

Section 3 Conclusions

Based on the evaluation provided in this demonstration, the RRPP BAB is in compliance with the location restrictions provided in §257.61 through §257.64 of the CCR rule. However, based on the evaluation provided in this demonstration, the RRPP BAB is not in compliance with the location restrictions provided in §257.60 of the CCR rule (due to placement of CCR within 5 ft of the uppermost aquifer) and is therefore subject to the closure requirements of 40 CFR 257.101(b)(1). A groundwater extraction system, installed around the perimeter of the BAB, has been operating continuously since March 2018 effectively capturing groundwater in the vicinity of the RRPP BAB CCR unit, and eliminates the potential for Appendix III and Appendix IV parameters to migrate from the RRPP BAB CCR unit. This demonstration will be placed into the operating record, posted to the publicly-accessible website, and government notifications provided.

Section 4 References

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Appendix A Monitoring Well Boring Logs

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_	River F	Roug	е	Wa	yne	Michigan	After Drilling:	Date	e/Time	6/14/1	6 11:0	4 ¥	Depth (ft bgs) 4.10
SAN	IPLE												
NUMBER AND TYPE	RECOVERY (%)	BLOW COUNTS	DEPTH IN FEET			LITHOLOGIO DESCRIPTIO			nscs	GRAPHIC LOG	WELL DIAGRAM	PID (PPM)	COMMENTS
						e to coarse sand, f			sw	(A. W.	11.		
ı				GRAVE	L WITH Se sand, i	dark gray (10YR 4 SAND mostly coars trace silt, dark gray	e gravel, some fin	9	GP	0.0		<1.0	Continuous sampling with 4-inch diameter casing from ground surface to terminus soil boring, over-drilled with 6-inch diameter casing to install monitoring well.
1 25	90		5-			e to coarse sand, y	ellowish brown		SW		1		
				SILT m	ostly silt,	dor, saturated. few fine to coarse dor, saturated, bric		/	ML		Н	-4.0	
Н					7.46	ay, few silt, trace fin			-		e d	<1.0	
1						y, no odor, moist.		,	CL				
2.1			10 -	PEAT I	igh orga	nic content, dark br	own (10YR 2/2),			11/1	11		
2 CS	90			natural	organic o	odor, moist, soft, wo	ood fragments pres	ent.		77.7	10.0	<1.0	
3 ST	100					ostly silt, few to little or, moist, medium			ML				
			15-	SILTY S (10YR 4	AND mo	ostly fine sand, few dor, moist to satura	to little silt, dark g ated.	ray	SP	1.11	H	<1.0	
4 CS	100			CAND			dest. ===: (40VD 4	41					
					, saturate	ne to coarse sand, o ed.	ualk glay (1011) 4	1),					
			20 -	İ					sw				
				İ							目		
5 CS	100											. <1.0	
00				Change	to shells	s present at 23.0 fe	et.						
						fine to coarse gra			GW	000			
			25-	CLAY	nostly cl	(10YR 4/1), no odo ay, trace silt, high p oist, soft.							
						25.0 feet below gro	ound surface.						

acilit	y/Projec						Date Drilling Started		Date Drilling		ted:	Page 1 of 2 Project Number:
-		E Ele	ctric (Company F		uge Power Plant	2/23/16			5/16	41.7.7.	231828.0005.000
rilling	g Firm:	ro Jie i	D-101-		Drilling M		Surface Elev. (ft)	12.00	evation (ft)	D. V. C. C.		(ft bgs) Borehole Dia. (in
orino	_		Orillin		ridge 25 fe	Sonic et south of basin.	580.28 Personnel	50	31.83		97.0 Equip	
				2855.24	riage, 20 to	or south of bushi.	Logged By - C. Sci Driller - A. Goldsmi			Drinnig	a cdoib	TSi 150cc
ivil T	own/Cit	ty/or Vil	lage:	County:		State:	Water Level Observ		Proces			Death (6 tox)
F	River I	Rouge	е	Wa	yne	Michigan	While Drilling: After Drilling:	Date/ Date/				Depth (ft bgs) Depth (ft bgs)
SAM	IPLE											
AND TYPE	RECOVERY (%)	BLOW COUNTS	DEPTH IN FEET			LITHOLOGI DESCRIPTION			nscs	GRAPHIC LOG	WELL DIAGRAM	COMMENTS
S	55		5-			ne to coarse sand, tra y (10YR 4/1), no odor		fine	sw			Continuous sampling with 4-inch diameter casing fro ground surface to terminu soil boring, over-drilled wit 6-inch diameter casing to install monitoring well.
			10-	_ dark gra	ay (10YF	T mostly fine to coars (4/1), no odor, moist, ostly clay, little to som	medium dense. e silt, trace to few	fine to	SW- SM CL- ML			
S	90		- - 15-	moist, s PEAT I natural SANDY	oft to me nigh orga organic SILT m	nedium plasticity, dark edium stiff. anic content, very dark odor, moist, soft to me ostly silt, little to some 8 4/1), no odor, moist,	t brown (10YR 2/2) edium stiff. e fine sand, nonpla	,	ML	<u> </u>		
			104	SILTYS	AND m	ostly fine sand, little s		₹ 4/1),	SM			
			20-	SAND	mostly fi	medium dense. ne sand, trace silt, da	rk gray (10YR 4/1)	, no	SP	111		
Ì				Change	turated, to wood	fragment present at SAND mostly fine to	21.0 feet.	to		پ ه		
				some s	and, dar	k gray (10YR 4/1), no ay, trace silt, high pla	odor, saturated, lo	ose.	GW	0		
S	100		25-			very soft.	ouoly, oroun (10	110,00,				
			30-	Change	to trace	fine gravel, trace coa	arse sand at 30.0 fo	eet.				
6	90		35-						CL			
			40-									

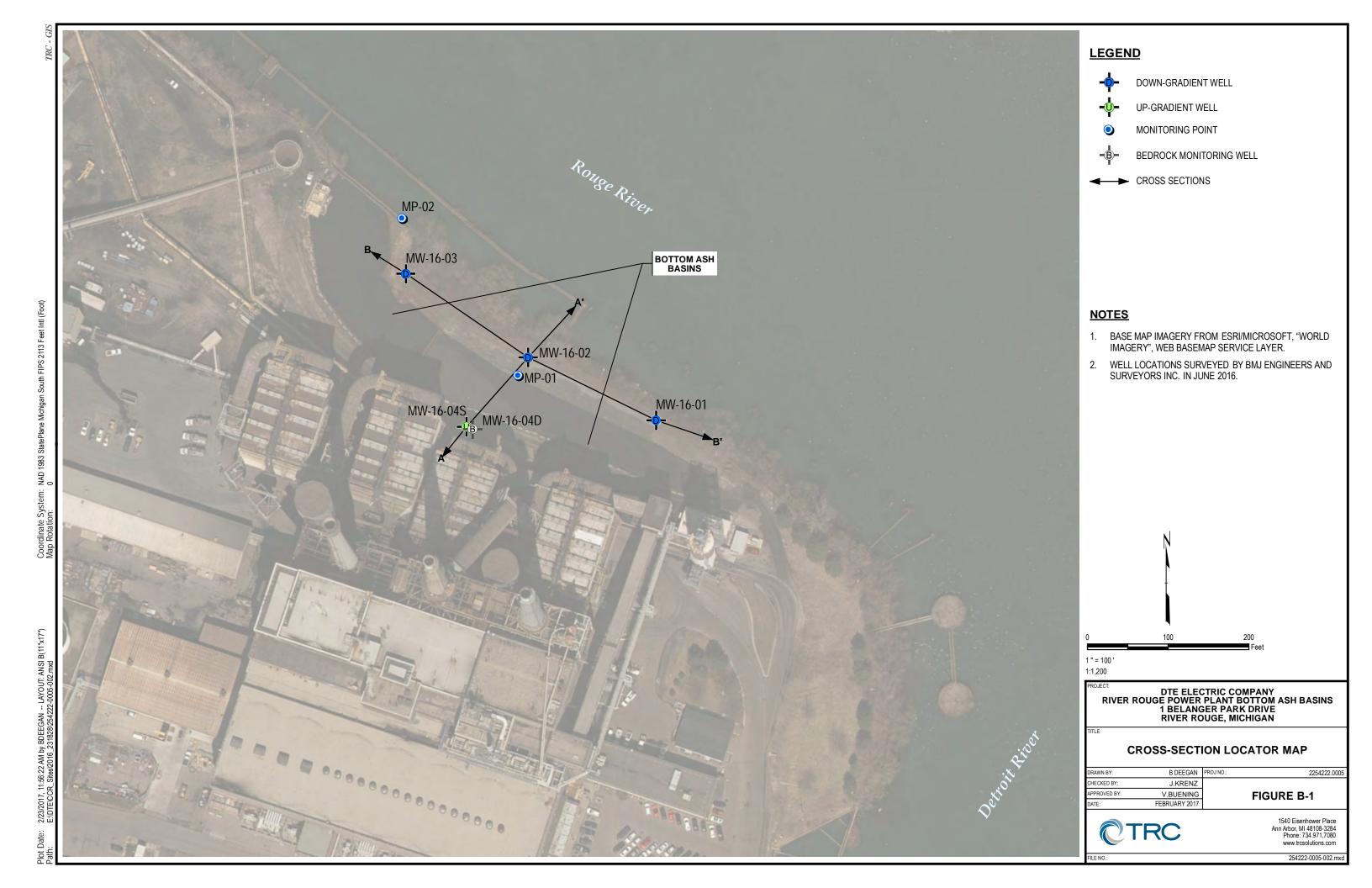
	2	TF	70	WELL CONSTRUCTION LOG	WE	LL N		W-16-04D Page 2 of 2
SAM	/PLE	c						
NUMBER AND TYPE	RECOVERY (%)	BLOW COUNTS	DEPTH IN FEET	LITHOLOGIC DESCRIPTION	nscs	GRAPHIC LOG	WELL DIAGRAM	COMMENTS
5 CS	90		45-		12.00			
6 ST	90		50					1
7 CS	100		55-					
			60-		CL			
8 CS	100		65 —					
			70-					
9 CS	75	Ŧ	75—					
			80-	Change to gray (10YR 5/1) at 79.0 feet.				
10 CS	80		85-	SILTY CLAY mostly clay, little to some silt, trace to few fine to coarse gravel, trace to few fine to coarse sand, low plasticity, dark gray (10YR 4/1), no odor, dry, hard.	CL- ML			
			90-	LIMESTONE white (10YR 8/1), dry, slight to moderate sulfur odor. Change to wet at 88.0 feet. Change to dark gray (10YR 4/1) at 90.0 feet.				
11 CS	50		95—					
		0	-	Change to white (10YR 8/1) at 96.0 feet. End of boring at 97.0 feet below ground surface.	-	H		

.,			70										Page 1 of 1
acilit	y/Proje						Date Drilling Starte	d:	Date I	Orilling		eted:	Project Number:
			ectric	Company		ge Power Plant	6/6/17	Tara		6/7			277472.0000.00
Orilling	g Firm:				Drilling Me		Surface Elev. (ft)	-	Elevatio	22 9626	Total		ft bgs) Borehole Dia.
Porine			Drillin		Incohed N of	Direct Push	579.9	5	83.01		Deillie	25.0	3.75
soring	Locat	on: E	ant buil	grassy berm ding.	located N of	parking lot SW of power	Personnel Logged By - C. So	ieszka			Drillir	ng Equip	oment:
				436.31		1.50	Driller - G. Geerlig					Geo	probe 7822DT
Civil T	own/C	ity/or V	illage:	County:		State:	Water Level Obser While Drilling:	C. 20 20 E 112 E 2	/Time	6/6/1	7 00:0	<u> </u>	Depth (ft bgs) _15.
F	River F	Rouge	Э	W	ayne	Michigan	After Drilling:			6/7/1	A STATE OF THE STA	-	
SAM	PLE												
AND TYPE	RECOVERY (%)	BLOW COUNTS	DEPTH IN FEET			LITHOLOGIC DESCRIPTION			nscs	GRAPHIC LOG	WELL DIAGRAM	PID (PPM)	COMMENTS
1-						AND mostly fine to co		(0)	3	ە پ	1		
			1 47		coarse sar r, dry, den	nd, trace silt, dark gra se.	yısh brown (10YR 4	12),	1.3	0()	1	<1	
P	70		3	SANDY	CLAY m	ostly clay, little to som				//			
			*			ace fine gravel, low p		ray	1	//,		<1	
			92-4			dor, moist, stiff to very						1	
-13			5-	0.5-inc	h thick inte	erval of brick at 4.5 fe	et.						
			-							//			
, 🖪			- 1						1	//	8 1	<1	
P	50									//	8 1		
										//	9 1	<1	
E	-		10-			VITH SAND mostly fire to coarse sand, trace			GM	O/Y			
			,,,			rd, dry, trace fine slag				Q(II)			
2 P 11 11 11 11 11 11 11 11 11 11 11 11 1				presen		k interval of peat, bla	ck (10VR 2/1) sligh	_/	CL			<1	
iP =	50			natural	odor, moi	st, soft.			OL	//			
Ē						D mostly clay, few to		r				<1	
					ow to med noist, med	ium plasticity, dark gr	ay (10YR 4/1), no	1	CL				
=			15-	¥ Grades	to SAND	Y CLAY mostly clay,	little to some fine to	· __		11			
			1	mediun	n sand, lo	w plasticity, light olive	gray (5Y 7/2), no o	dor, į				9	
4 B	60		<u> </u>		medium st	ள. stly fine sand, little sil	dark gray (10YR	'		批准			
4 SP	-		-	4/1) wit	th yellowis	h brown (10YR 5/6) i							
員	8 1		1 4	saturat	ed, dense				SM	111			
			20 -	Change	e to trace	fine to coarse gravel	at 20 0 feet			清洁			
			4	Chang	e to trace	illie to coarse graver	at 20.0 loct.					}	
目。									1113		:目		
5 P	70					AND mostly fine to c				00	i e		
					little medi r, saturate	um to coarse sand, d	ark gray (10YR 4/1)	,	GW	0 ()			
Ē			25 —					-	CL	o.			
				mediur moist, i	n plasticity medium st	y, trace fine to mediu r, dark grayish brown iff. 25.0 feet below grou	(10YR 4/2), no odd	or,	, or				
			i	777	with	9,00							
												_	

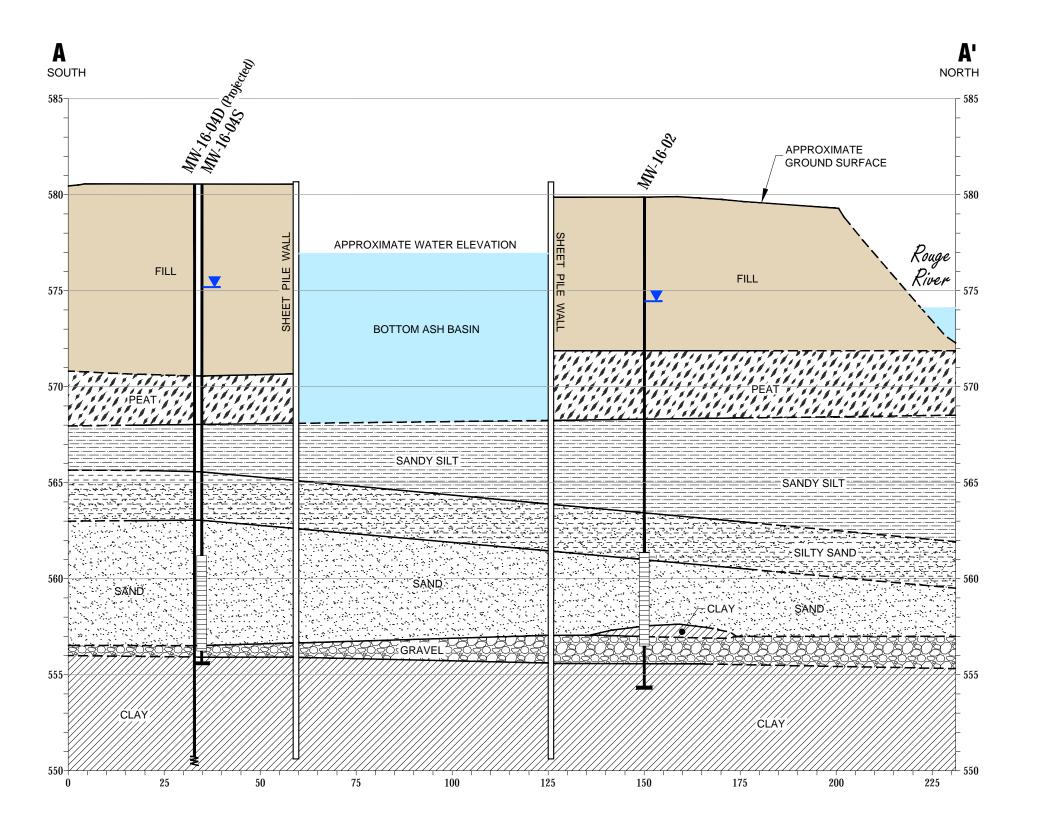
Checked By: T. Hess

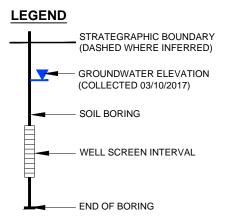
													Page 1 of 1
acility/	Proje	ct Nam	e:				Date Drilling Started	:-	Date I	Drilling (Compl		Project Number:
	DT	E Ele	ectric	Company	River Rou	ge Power Plant	6/14/17			6/14	/17		277472,0000.000
rilling			* E-111-1-2-2	51 E/6 T (F/545) / 5	Drilling Me		Surface Elev. (ft)	TOC	Elevatio	on (ft)	Total	Depth	(ft bgs) Borehole Dia. (i
	St	earns	Drillin	ng	Direct P	ush/Hollow Stem Auger	580.0	5	83.0	5		25.0	8/3.75/2
oring L		on: N	of entra	ance road, ne		Park entrance, adjacent to S	Personnel	1.763			Drillin	g Equi	pment:
1: 283:	337.3	100		ooundary. 939.92			Logged By - C. Scientification of the Control of th					Geo	probe 7822DT
4		ty/or Vi		1.1313141		State:	Water Level Observ		,				, p. 1020 1 0220 1
Riv	ver F	Rouge		10/	ayne	Michigan	While Drilling: After Drilling:			6/14/			
SAMPI		touge		VV	ayrıc	Wildingan	Arter Dilling.	Date	rime	0/13/	17 11.5		
AND TYPE	RECOVERY (%)	SLOW COUNTS	DEPTH IN FEET			LITHOLOGIC DESCRIPTION			nscs	GRAPHIC LOG	WELL DIAGRAM	PID (PPM)	COMMENTS
-	_		_	SILTY S	AND mo	stly fine to medium sand	, little to some		SM	HELL	Í,	-	
H			1	¬ silt, trac		ne gravel, brown (10YR		, __	19700	76 P \$		- 22	Soil sample and duplicate
	90		1	\loose.	AV MAT	H SAND AND GRAVEL I	mostly olay	/	CL- ML			<1	sample collected (0-2') at 1100.
	80		5 —	coarse 3/3), no fragmer COAL V coal frag (10YR 2 Change pore wa (10YR 2 CLAY r (10YR 3 CLAY r (10YR 2 Change 11.0 fee	gravel, no odor, dry nts preser WITH SLAG gments, fe 2/1), no od to slight ater at 5.0 mostly clay (1/1), no od nostly clay (2/1), no od to no silt et o no silt et.	G FILL mostly fine to me ew to little slag fragments dor, saturated, medium o hydrocarbon odor, very	rk brown (10YR cinder, and wood edium sand sized s, trace silt, black dense. slight sheen on ticity, dark gray dark brown m plasticity, black 5/1), medium stiff		CL		Services Services		Hollow stem augers set a 10.0 feet below ground surface prior to drilling through confining clay un
	85			sand, n saturate	o plasticity ed, stiff.	mostly silt, few to little fi y, gray (10YR 5/1), no od n (10YR 5/3) at 18.5 feet	dor, moist to		ML				
	90		25—	5/1), no 0.25-ind sand, fe odor, sa Change with ligh stiff at 2 Change	odor, moch thick salew to little aturated, of the trace o	y, trace silt, medium plas bist, medium stiff. and with gravel seam, me fine gravel, yellowish bro dense at 20.5 feet. coarse sand to fine grave brown (5YR 6/4) mottles medium stiff at 24.0 fee 25.0 feet below ground s	ostly fine to coars own (10YR 5/6), n el, gray (10YR 5/1 s, dry to moist, ve t.	e 10	CL				

Appendix B Cross Sections

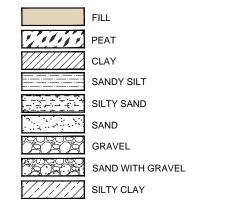


GENERALIZED GEOLOGIC CROSS-SECTION A-A'



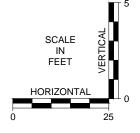


Lithology Key



NOTE

1. ROUGE RIVER WATER ELEVATION AT 574.10' IS APPROXIMATE.



DTE ELECTRIC COMPANY
COAL COMBUSTIBLE RESIDUALS
RIVER ROUGE POWER PLANT
RIVER ROUGE, MICHIGAN

TITLE:

GENERALIZED GEOLOGIC CROSS-SECTION A-A'

DRAWN BY:	D.STEHLE	PROJ NO.:
CHECKED BY:	J.KRENZ	
APPROVED BY:	V.BUENING	F
DATE:	MAY 2017	

FIGURE B-2

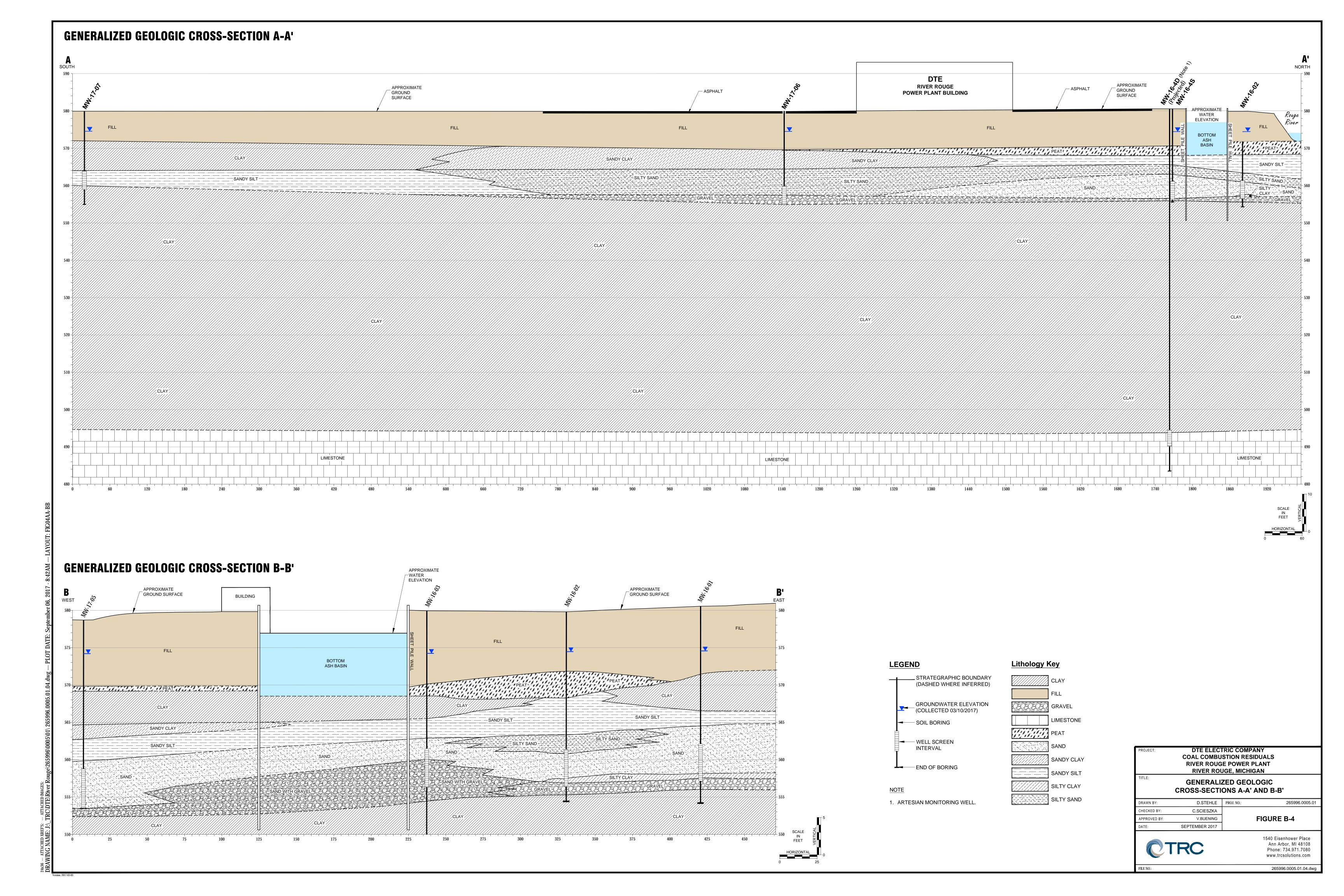


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265996.0005

265996.0005.01.XS AA-BB.dwg

LEGEND GENERALIZED GEOLOGIC CROSS-SECTION B-B' STRATEGRAPHIC BOUNDARY (DASHED WHERE INFERRED) - GROUNDWATER ELEVATION (COLLECTED 03/10/2017) APPROXIMATE -WATER **ELEVATION** - SOIL BORING APPROXIMATE B' B GROUND SURFACE - WELL SCREEN INTERVAL EAST WEST - END OF BORING FILL Lithology Key FILL 575-575 FILL PEAT воттом ASH BASIN CLAY SANDY SILT SILTY SAND 570-SAND GRAVEL SAND WITH GRAVEL SILTY CLAY SANDY SILT SANDY SILT SILTY SAND SILTY SAND SAND SAND SILTY CLAY GRAVEL SCALE IN FEET ĆĽÁÝ CLAY HORIZONTAL DTE ELECTRIC COMPANY 25 125 175 225 250 COAL COMBUSTIBLE RESIDUALS RIVER ROUGE POWER PLANT RIVER ROUGE, MICHIGAN **GENERALIZED GEOLOGIC CROSS-SECTION B-B'** D.STEHLE PROJ NO.: 265996.0005 DRAWN BY: CHECKED BY: J.KRENZ V.BUENING FIGURE B-3 APPROVED BY: MAY 2017 1540 Eisenhower Place Ann Arbor, MI 48108 Phone: 734.971.7080 www.trcsolutions.com 265996.0005.01.XS AA-BB.dwg



Appendix C National Wetland Inventory Map

Wetlands Map Viewer



August 17, 2018

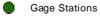
Part 303 Final Wetlands Inventory

Wetlands as identified on NWI and MIRIS maps

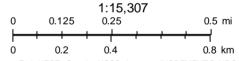
Soil areas which include wetland soils

// Wet

Wetlands as identified on NWI and MIRIS maps and soil areas which include wetland soils



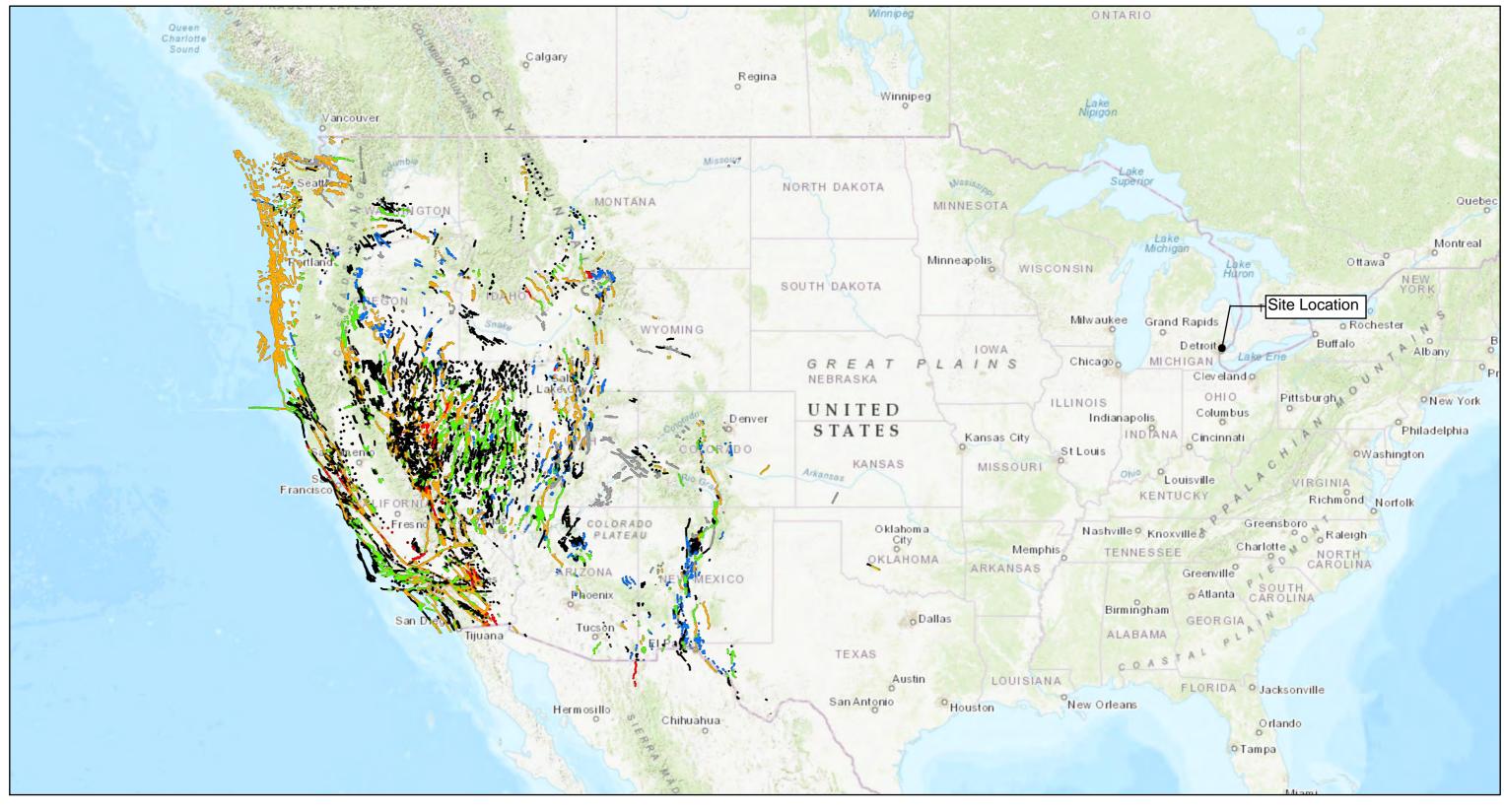
National Wetlands Inventory 2005



Sources: Esri, HERE, Garmin, USGS, Intermap, INCREMENT P, NRCan, Esri Japan, METI, Esri China (Hong Kong), Esri Korea, Esri (Thailand), NGCC, ⊚ OpenStreetMap contributors, and the GIS User Community

Appendix D U.S. Quaternary Faults and Folds Map

US Quaternary Faults and Fdds



9/7/2018 32039PM

Quaterrary faults

- unspecified age, well constrained location
- -- unspecified age, moderately constrained location
- " unspecified age, inferred location
- undifferentiated Quaternary (< 130,000 years), well constrained location
- " undifferentiated Quaterrary (< 130,000 years), moderately constrained location ___
- undifferentiated Quaterrary (< 130,000 years), inferred location
- middle and late Quaterrary (< 1.6 million years), well constrained location
- middle and late Quaterrary (< 1.6 million years), moderately constrained location
- " middle and late Quaterrary (< 1.6 million years), inferred location
- latest Quaternary (<15,000 years), well constrained location
 - latest Quaternary (<15,000 years), moderately constrained location

" latest Quaternary (<15,000 years), inferred location

late Quaterrary (< 130,000 years), well constrained location

1:18,489,298 O 175 350 700mi O 275 550 1,100km

Sources Esti, HERE, Garmin, Intermap, increment P.Corp., GEBCO, USGS, FAO, NPS, NRCAN, GeoBæse, IGN, Kadaster NL, Ordhance Survey, Esti Japan, METI, Esti China (Hong Kong), svisstopo, © OpenStreetMap contributors, and the GIS User Community, USGS

Appendix E U.S. Seismic Design Maps and Calculations

U.S. Geological Survey - Earthquake Hazards Program

Due to insufficient resources and the recent development of similar web tools by third parties, this spring the USGS will be streamlining the two U.S. Seismic Design Maps web applications, including the one below. Whereas the current applications each interact with users through a graphical user interface (GUI), the new web services will receive the inputs (e.g. latitude and longitude) in the form of a web address and return the outputs (e.g. S_{DS} and S_{D1}) in text form, without supplementary graphics. Though designed primarily to be read by the aforementioned third-party web GUIs, the text outputs are also human-readable. To preview the new web services, please click here. Step-by-step instructions for using one of these web services, namely that for the recently published 2016 ASCE 7 Standard, are posted here.

RRPP BAB - Seismic Impact Zone

Latitude = 42.274°N, Longitude = 83.112°W

Location



Reference Document

2015 NEHRP Provisions

Site Class

D (default): Stiff Soil

Risk Category

I or II or III

 $S_S = 0.105 g$

 $S_{MS} = 0.169 g$

 $S_{DS} =$

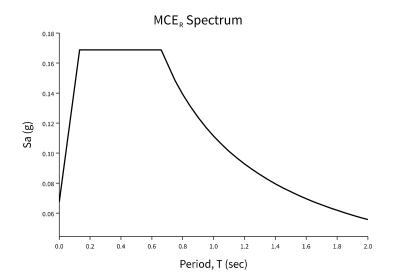
0.113 g

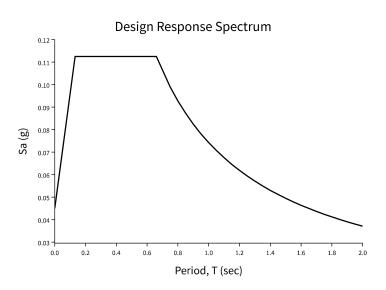
0.074 g

S₁ = 0.046 g

 $S_{M1} = 0.111 g$

S_{D1} =





Mapped Acceleration Parameters, Long-Period Transition Periods, and Risk Coefficients

Note: The S_S and S_1 ground motion maps provided below are for the direction of maximmum horizontal spectral response acceleration. They have been converted from corresponding geometric mean ground motions computed by the USGS by applying factors of 1.1 (to obtain S_S) 1.3 (to obtain S_1).

- FIGURE 22-1 S_S Risk-Targeted Maximum Considered Earthquake (MCE_R) Ground Motion Parameter for the Conterminous United States for 0.2 s Spectral Response Acceleration (5% of Critical Damping), Site Class B
- <u>FIGURE 22-2 S₁ Risk-Targeted Maximum Considered Earthquake (MCE_R) Ground Motion Parameter</u>
 <u>for the Conterminous United States for 1.0 s Spectral Response Acceleration (5% of Critical Damping), Site Class B</u>
- FIGURE 22-9 Maximum Considered Earthquake Geometric Mean (MCE_G) PGA, %g, Site Class B for the Conterminous United States
- FIGURE 22-14 Mapped Long-Period Transition Period, T_L (s), for the Conterminous United States
- FIGURE 22-18 Mapped Risk Coefficient at 0.2 s Spectral Response Period, CRS
- FIGURE 22-19 Mapped Risk Coefficient at 1.0 s Spectral Response Period, C_{R1}

Site Class

The authority having jurisdiction (not the USGS), site-specific geotechnical data, and/or the default has classified the site class as Site Class, based on the site soil properties in accordance with Chapter 20.

Table 20.3-1 Site Classification

Site Class	- v _S	\overline{N} or \overline{N}_{ch}	- s _u
A. Hard Rock	>5,000 ft/s	N/A	N/A
B. Rock	2,500 to 5,000 ft/s	N/A	N/A
C. Very dense soil and soft rock	1,200 to 2,500 ft/s	>50	>2,000 psf
D. Stiff Soil	600 to 1,200 ft/s	15 to 50	1,000 to 2,000 psf
E. Soft clay soil	<600 ft/s	<15	<1,000 psf
	 Any profile with more than 1 Plasticity index PI > 20 Moisture content w ≥ 40 Undrained shear streng 	0% <u>,</u> and	he characteristics:
F. Soils requiring site response analysis in accordance with Section 21.1		See Section 20.3.1	
	1ft/s = 0.3048 m/s 1lb/ft ² = 0.047	9 kN/m ²	

Site Coefficients and Risk-Targeted Maximum Considered Earthquake (MCE_R) Spectral Response Acceleration Parameters

Risk-targeted Ground Motion (0.2 s)

 $C_{RS}S_{SUH} = 0.946 \times 0.112 = 0.105 g$

Deterministic Ground Motion (0.2 s)

 $S_{SD} = 1.500 g$

 $S_S \equiv$ "Lesser of $C_{RS}S_{SUH}$ and S_{SD} " = 0.105 g

Risk-targeted Ground Motion (1.0 s)

 $C_{R1}S_{1UH} = 0.907 \times 0.051 = 0.046 g$

Deterministic Ground Motion (1.0 s)

 $S_{1D} = 0.600 g$

 $S_1 \equiv$ "Lesser of $C_{R1}S_{1UH}$ and S_{1D} " = 0.046 g

Table 11.4-1: Site Coefficient Fa

	Spectral Reponse Acceleration Parameter at Short Period							
Site Class	S _S ≤ 0.25	S _S = 0.50	S _S = 0.75	S _S = 1.00	S _S = 1.25	S _S ≥ 1.50		
А	0.8	0.8	0.8	0.8	0.8	0.8		
B (measured)	0.9	0.9	0.9	0.9	0.9	0.9		
B (unmeasured)	1.0	1.0	1.0	1.0	1.0	1.0		
С	1.3	1.3	1.2	1.2	1.2	1.2		
D (determined)	1.6	1.4	1.2	1.1	1.0	1.0		
D (default)	1.6	1.4	1.2	1.2	1.2	1.2		
E	2.4	1.7	1.3	1.2 *	1.2 *	1.2 *		
F	See Section 11.4.7							

^{*} For Site Class E and $S_S \ge 1.0$ g, see the requirements for site-specific ground motions in Section 11.4.7 of the 2015 NEHRP Provisions. Here the exception to those requirements allowing F_a to be taken as equal to that of Site Class C has been invoked.

Note: Use straight-line interpolation for intermediate values of S_S.

Note: Where Site Class B is selected, but site-specific velocity measurements are not made, the value of F_a shall be taken as 1.0 per Section 11.4.2.

Note: Where Site Class D is selected as the default site class per Section 11.4.2, the value of F_a shall not be less than 1.2 per Section 11.4.3.

For Site Class = D (default) and $S_S = 0.105 g$, $F_a = 1.600$

Table 11.4-2: Site Coefficient F_v

	Spectral Response Acceleration Parameter at 1-Second Period						
Site Class	S ₁ ≤ 0.10	S ₁ = 0.20	S ₁ = 0.30	S ₁ = 0.40	S ₁ = 0.50	S ₁ ≥ 0.60	
А	0.8	0.8	0.8	0.8	0.8	0.8	
B (measured)	0.8	0.8	0.8	0.8	0.8	0.8	
B (unmeasured)	1.0	1.0	1.0	1.0	1.0	1.0	
С	1.5	1.5	1.5	1.5	1.5	1.4	
D (determined)	2.4	2.2 1	2.0 ¹	1.9 ¹	1.8 1	1.7 ¹	
D (default)	2.4	2.2 ¹	2.0 ¹	1.9 ¹	1.8 1	1.7 1	
Е	4.2	3.3 ¹	2.8 1	2.4 ¹	2.2 1	2.0 ¹	
F	See Section 11.4.7						

 $^{^{1}}$ For Site Class D or E and S₁ ≥ 0.2 g, site-specific ground motions might be required. See Section 11.4.7 of the 2015 NEHRP Provisions.

Note: Use straight-line interpolation for intermediate values of S₁.

Note: Where Site Class B is selected, but site-specific velocity measurements are not made, the value of F_v shall be taken as 1.0 per Section 11.4.2.

For Site Class = D (default) and $S_1 = 0.046 \text{ g}$, $F_V = 2.400 \text{ m}$

Site-adjusted MCE_R (0.2 s)

$$S_{MS} = F_a S_S = 1.600 \times 0.105 = 0.169 g$$

Site-adjusted MCE_R (1.0 s)

$$S_{M1} = F_V S_1 = 2.400 \times 0.046 = 0.111 g$$

Design Spectral Acceleration Parameters

Design Ground Motion (0.2 s)

$$S_{DS} = \frac{2}{3} S_{MS} = \frac{2}{3} \times 0.169 = 0.113 g$$

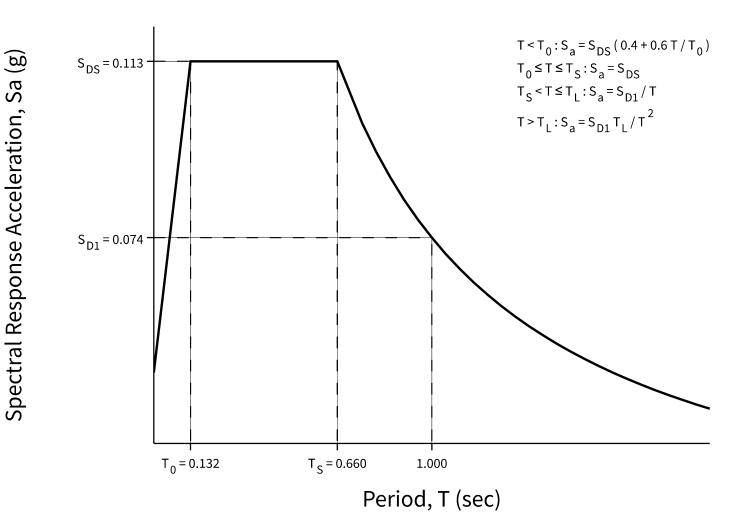
Design Ground Motion (1.0 s)

$$S_{D1} = \frac{2}{3} S_{M1} = \frac{2}{3} \times 0.111 = 0.074 g$$

Design Response Spectrum

Long-Period Transition Period = T_L = 12 s

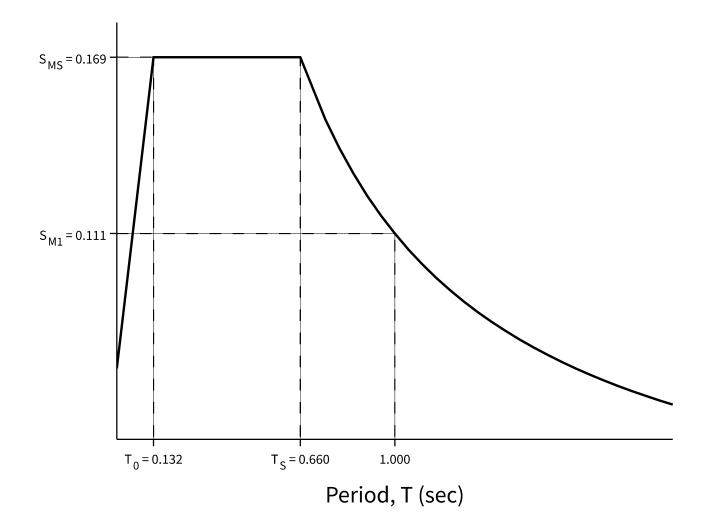
Figure 11.4-1: Design Response Spectrum



MCE_R Response Spectrum

The MCE_R response spectrum is determined by multiplying the design response spectrum above by 1.5.





Additional Geotechnical Investigation Report Requirements for Seismic Design Categories D through F

Table 11.8-1: Site Coefficient for F_{PGA}

	Mapped MCE Geometric Mean (MCE _G) Peak Ground Acceleration						
Site Class	PGA ≤ 0.10	PGA = 0.20	PGA = 0.30	PGA = 0.40	PGA = 0.50	PGA ≥ 0.60	
А	0.8	0.8	0.8	0.8	0.8	0.8	
B (measured)	0.9	0.9	0.9	0.9	0.9	0.9	
B (unmeasured)	1.0	1.0	1.0	1.0	1.0	1.0	
С	1.3	1.2	1.2	1.2	1.2	1.2	
D (determined)	1.6	1.4	1.3	1.2	1.1	1.1	
D (default)	1.6	1.4	1.3	1.2	1.2	1.2	
Е	2.4	1.9	1.6	1.4	1.2	1.1	
F	See Section 11.4.7						

Note: Use straight-line interpolation for intermediate values of PGA

Note: Where Site Class D is selected as the default site class per Section 11.4.2, the value of F_{pga} shall not be less than 1.2.

For Site Class = D (default) and PGA = 0.053 g, $F_{PGA} = 1.600$

Mapped MCE_G

PGA = 0.053 g

Site-adjusted MCE_G

 $PGA_{M} = F_{PGA}PGA = 1.600 \times 0.053 = 0.085 g$